1. Estimate the ultimate bearing capacity of a square isolated footing $(2 \mathrm{~m} \times 2 \mathrm{~m})$ in compacted
sand (having $\varphi=32^{\circ}$ ) that overlies loose sand (having $\varphi=22^{\circ}$ ).
Depth of foundation $=1.5 \mathrm{~m}$; The thickness of layer- $1=3 \mathrm{~m}$.
Assume that the saturated unit weights of dense sand and loose sand are $18 \mathrm{kN} / \mathrm{m}^{3}$ and 15 $\mathrm{kN} / \mathrm{m}^{3}$. The water table is at 1.5 m below the GL.
2. Determine the allowable bearing capacity of a strip footing in sand (having $\varphi=32^{\circ}$ ) that overlies soft clay (having $\mathrm{c}=20 \mathrm{kPa}$ ).
Depth of foundation $=2 \mathrm{~m}$; The thickness of layer $-1=3 \mathrm{~m}$.
Assume that the saturated unit weights of sand and soft clay are $18 \mathrm{kN} / \mathrm{m}^{3}$ and $19.2 \mathrm{kN} / \mathrm{m}^{3}$.
The water table is at 1.5 m below the GL.
3. According to the soil exploration report, the upper layer is found homogeneous and
extends up to 3 m below the ground level. The ground water table is located at 1.5 m below GL. The data of the soil layers is as follows:
Given data: $\gamma_{\mathrm{sat}}=17.2 \mathrm{kN} / \mathrm{m}^{3}$;
Layer-1: $\varphi_{1}=20^{\circ}$
Layer-2: $\varphi_{2}=33^{\circ}$
(a) Estimate the allowable bearing capacity of a 2 m wide strip footing, placed at a depth 1.5 m below the ground level. Provide a factor of safety equal 2. Use Terzaghi's bearing capacity equation and apply Hanna's design charts for modified bearing capacity factors.
(b) Recalculate the allowable bearing capacity of the same footing if the foundation is placed at a depth of 2.6 m .
Compare between the allowable bearing capacities obtained in (a) and (b).
4. (i) The structural distress is observed for the first time in the $12^{\text {th }}$ year after construction due to some problem related to foundation. Detect the possible cause(s) of the problem. How can these problems be identified before construction?
(ii) In a standard penetration test, the recorded blow counts were $2 / 2 / 1$ for three consecutive 150 mm penetration. Assess the soil strength. It was possible to collect undisturb sample from the standard sampler.
(iii) In a standard penetration test, borehole diameter $=120 \mathrm{~mm}$; sampling method (Standard); $\gamma_{\mathrm{sat}}=17.5 \mathrm{kN} / \mathrm{m}^{3}$; rod length $=4 \mathrm{~m}$. Water table is at the ground level.
(a) Determine the hammer efficiency, if the recorded blow counts are $3 / 6 / 5$ for three consecutive 150 mm penetrations and $(\mathrm{N})_{60}=9$.
(b) Determine the missing blow count ' X ' if the recorded blow counts are $4 / \mathrm{X} / 7$ for three consecutive 150 mm penetrations. The hammer efficiency is 0.55 and $(\mathrm{N})_{60}=17$.
(c) At a depth of 5 m , the recorded blow counts are $4 / 6 / 6$ for three consecutive 150 mm penetrations. Calculate $\left(\mathrm{N}_{1}\right)_{60}$. Apply the hammer efficiency determined in (a).

Additional Information:

| $\phi$ | $N_{i}$ | $N_{q}$ | $N_{\mathrm{y}}$ |
| :---: | :---: | :---: | :---: |
| 17 | 12.34 | 4.77 | 1.66 |
| 18 | 13.10 | 5.26 | 2.00 |
| 19 | 13.93 | 5.80 | 2.40 |
| 20 | 14.83 | 6.40 | 2.87 |
| 21 | 15.82 | 7.07 | 3.42 |
| 22 | 16.88 | 7.82 | 4.07 |
| 23 | 18.05 | 8.66 | 4.82 |
| 24 | 19.32 | 9.60 | 5.72 |
| 25 | 20.72 | 10.66 | 6.77 |
| 26 | 22.25 | 11.85 | 8.00 |
| 27 | 23.94 | 13.20 | 9.46 |
| 28 | 25.80 | 14.72 | 11.19 |
| 29 | 27.86 | 16.44 | 13.24 |
| 30 | 30.14 | 18.40 | 15.67 |
| 31 | 32.67 | 20.63 | 18.56 |
| 32 | 35.49 | 23.18 | 22.02 |
| 33 | 38.64 | 26.09 | 26.17 |

$$
\begin{aligned}
& \mathrm{S}_{\mathrm{c}}=1+0.2(\mathrm{~B} / \mathrm{L}) \\
& \mathrm{S}_{\mathrm{q}}=1+0.2(\mathrm{~B} / \mathrm{L}) \\
& \mathrm{S}_{\gamma}=1+0.4(\mathrm{~B} / \mathrm{L}) \\
& \mathrm{d}_{\mathrm{c}}=1+0.2\left(\mathrm{D}_{\mathrm{f}} / \mathrm{B}\right) \\
& \mathrm{d}_{\mathrm{q}}=\mathrm{d}_{\gamma}=1+0.1 \mathrm{D}_{\mathrm{f}} \tan \left(45^{\circ}+\varphi / 2\right)
\end{aligned}
$$

| Factor | Equipment Variables | Value |
| :--- | :--- | :--- |
| Borehole diameter | $65-115 \mathrm{~mm}(2.5-4.5 \mathrm{in})$ | 1.00 |
| factor. $C_{B}$ | $150 \mathrm{~mm}(6 \mathrm{in})$ | 1.05 |
|  | $200 \mathrm{~mm}(8 \mathrm{in})$ | 1.15 |
| Sampling method | Standard sampler | 1.00 |
| factor, $C_{S}$ | Sampler without liner <br> (not recommended $)$ | 1.20 |
| Rod length factor, $C_{R}$ | $3-4 \mathrm{~m}(10-13 \mathrm{ft})$ | 0.75 |
|  | $4-6 \mathrm{~m}(13-20 \mathrm{ft})$ | 0.85 |
|  | $6-10 \mathrm{~m}(20-30 \mathrm{ft})$ | 0.95 |
|  | $>10 \mathrm{~m}(>30 \mathrm{ft})$ | 1.00 |



Fig. 3. Punching shear parameter.



UNDRAINED SHEAR STRENGTH OF CLAY (KPa)
Fig. 4. Coefficients of punching shear: (a) $\phi_{1}=50^{\circ}$; (b) $\phi_{1}=45^{\circ}$; (c) $\phi_{1}=40^{\circ}$.




FIG. 8.-Design Chart for Determining Coefficient of Punching Shear Resistance, $K_{s}$

# University of Asia Pacific Department of Civil Engineering Mid Semester Examination Fall 2016 Program: MCE 

Course No: CE 6301

Course Title: Theory of Water Treatment
Time: 1.0 hour

1. (a) Water pollution is all about quantity -- describe this statement. ..... [05]
(b) Derive Stokes' Law for Type I sedimentation. ..... [15]
2. (a) What are the operational tests that should be performed during aeration process? ..... [05]
(b) Discuss with applications the different types of settling mechanisms of particles during sedimentation. ..... [15]
3. (a) Mention the Camp's assumptions of an Ideal Sedimentation Basin. ..... [05](b) Discuss the aeration process of water treatment with its objectives.[15]
4. (a) Define adsorption, absorption, adsorbate, adsorbent. ..... [10]
(b) Mention the factors that affecting adsorption process in water treatment. ..... [05](c) Sleepy Hollow has an existing horizontal-flow sedimentation tank with anoverflow rate of $20 \mathrm{~m}^{3} / \mathrm{d}^{*} \mathrm{~m}^{2}$. What percentage removal should be expected foreach of the following particle settling velocities in an ideal sedimentation tank:$0.1 \mathrm{~mm} / \mathrm{s}, 0.2 \mathrm{~mm} / \mathrm{s}$, and $1 \mathrm{~mm} / \mathrm{s}$ ?[05]

# University of Asia Pacific <br> Department of Civil Engineering <br> Mid Semester Examination Fall 2016 <br> Program: MCE 

Course Title: Structural Design of Pavement
Course Code: CE 6505
Time: 1 Hour
Full Marks: 20

1. Answer any Five $(5 \times 2=10)$
A. Where do most critical stress, strain and deflection occur when the load is applied over a single circular loaded area of flexible pavement?
B. Write 2 assumptions for analysing flexible pavement based on Burmister Layered theory.
C. Name the factors upon which the stresses on two layer systems depend.
D. How will you compute stress and strain in asphalt layer by 2 layers pavement analysis if the pavement is composed of three layers (e.g., as asphalt surface course, a granular base course and a subgrade)?
E. How do daily and seasonal variations of temperature affect Rigid Pavement?
F. Define: Modulus of Subgrade Reaction and Radius of Relative Stiffness
G. Write the causes of stresses in Slab. What are the methods available for calculating wheel load stresses and deflections in Rigid Pavement?
2. Answer any One $(10 \times 1=10)$
A. i) A full-depth asphalt pavement 8 in . $(203 \mathrm{~mm})$ thick subjected to a single-whee 1 load of $9000 \mathrm{lb}(40 \mathrm{kN})$ having contact pressure $67.7 \mathrm{psi}(467 \mathrm{kPa})$. If the elastic modulus of the asphalt layer is $150,000 \mathrm{psi}(1.04 \mathrm{GPa})$ and that of the subgrade is $15,000 \mathrm{psi}(104 \mathrm{MPa})$, determine the critical tensile strain in the asphalt layer.
ii) Given a concrete pavement with a joint spacing of $25 \mathrm{ft}(7.6 \mathrm{~m})$ and a coefficient of friction of 1.5 , determine the stress in concrete due to friction.
For the same pavement $\Delta T=60^{\circ} \mathrm{F}\left(33^{\circ} \mathrm{C}\right)$, at $=5.5 \times 10^{-6} /{ }^{\circ} \mathrm{F}\left(9.9 \times 10^{-6} /{ }^{\circ} \mathrm{C}\right)$, $e=1.0 \times 10^{-4}$, $C=0.65$, and the allowable joint openings for undoweled and doweled joints are 0.05 and 0.25 in. ( 1.3 and 6.4 mm ), respectively, determine the maximum allowable joint spacing.
A. i) Design size and spacing of dowel bars at an expansion joint of concrete pavement of thickness 25 cm . Given the radius of relative stiffness of 80 cm . design wheel load 5000 kg . Load capacity of the dowel system is 40 percent of design wheel load. Joint width is 2.0 cm and the permissible stress in shear, bending and bearing stress in dowel bars are 1000,1400 and $100 \mathrm{~kg} / \mathrm{cm}^{2}$ respectively.
ii) A homogeneous half-space subjected to two circular loads, each 10 in . ( 254 mm ) in diameter and spaced at 20 in . ( 508 mm ) on centers. The pressure on the circular area is 50 psi $(345 \mathrm{kPa})$. The half-space has elastic modulus $10,000 \mathrm{psi}(69 \mathrm{MPa})$ and Poisson ratio 0.5 . Determine the vertical stress, strain, and deflection at point A, which is located 10 in . (254 mm ) below the center of one circle.

## Equations:

$$
\begin{gathered}
\sigma_{\mathrm{c}}=\frac{\gamma_{\mathrm{c}} L f_{\mathrm{a}}}{2} \\
\Delta L=C L\left(\alpha_{\mathrm{t}} \Delta T+\epsilon\right)
\end{gathered}
$$

## University of Asia Pacific Department of Civil Engineering Mid Semester Examination Fall 2016 <br> Program: MCE

Course Title: Structural Design of Pavement Time: 1 Hour

Course Code: CE 6505
Full Marks: 20

$$
\begin{aligned}
\epsilon_{z} & =\frac{1}{E}\left[\sigma_{z}-v\left(\sigma_{r}+\sigma_{t}\right)\right] \\
\epsilon_{r} & =\frac{1}{E}\left[\sigma_{r}-v\left(\sigma_{t}+\sigma_{z}\right)\right] \\
L_{d}=5 d \sqrt{\frac{F_{f}}{F_{b}} \frac{\left(L_{d}+1.5 \delta\right)}{\left(L_{d}+8.8 \delta\right)}} & \epsilon_{t}
\end{aligned}=\frac{1}{E}\left[\sigma_{t}-v\left(\sigma_{z}+\sigma_{r}\right)\right], ~ \$
$$

# University of Asia Pacific <br> Department of Civil Engineering <br> Mid Semester Examination Fall 2016 

Course No: CE 6204
Time: 1 Hour
Course Title: Repair and Strengthening of Concrete Structures

1. (a) Write short notes on the cracking due to:
(i) Plastic shrinkage,
(ii) Chemical reaction,
(iii) Construction overload,
(iv) Design and detailing.
(b) Identify the problems from the images shown below:

(i)

(iii)

(ii)

(iv)
2. (a) What do you understand by evaluation of cracking? Explain the process of crack sealing.
(b) Describe the crack evaluation using ultrasonic pulse velocity technique.
(c) What considerations needs to be taken during corrosion potential test?
(d) Write short notes on (i) Mechanical movement indicator, (ii) Pachometer, (iii) Flexible shaft Fiberscope
3. A 25 years old residential building slab has been cracked due to steel corrosion. It is necessary to identify the causes of crack. Determine the concrete failure probability of the slab due to the carbonation. Also determine the corrosion propagation time.
[Given: $\mathrm{CO}_{2}$ content in the atmosphere is $0.035 \%$, cement content $=340 \mathrm{~kg} / \mathrm{m}^{3}$, water content $=170 \mathrm{~kg} / \mathrm{m}^{3}$ aggregate to cement ratio $=4.5, \rho_{c}=94 l b / f t^{3}, \rho_{a}=89 l b / f t^{3}, n=2.5$. Slab was cured for seven days. The measurement was taken in July 2016]


Figure 1. Monthly relative humidity and temperature of Dhaka city

| Structural <br> Element Type | Clear cover (mm) <br> (ACI 318) |
| :--- | :---: |
| Slab | 19 |
| Beam | 37.5 |
| Column | 37.5 |

$$
k_{c o n}=350 \frac{\rho_{c}}{\rho_{w}}\left(\frac{\frac{w}{c}-0.30}{1+\frac{\rho_{c}}{\rho_{w}} \frac{w}{c}}\right) \sqrt{1+\frac{\rho_{c}}{\rho_{w}} \frac{w}{c}+\frac{\rho_{c}}{\rho_{a}} \frac{a}{c}}
$$

# University of Asia Pacific <br> Department of Civil Engineering <br> Mid-term Examination Fall 2016 

Time: 1 (One) Hour
Course Title: Advance Theory and Design of Steel Structure

There are 2 questions in this section. Answer all the questions.
Assume reasonable values for any missing data

1. (a) Why is the true maximum stress in a steel tension coupon higher than the engineering stress near rupture
(b) What do the coefficients " $U$ " and " 0.6 " represent in the block shear equation in AISC (be specific)?
(c) Compute the yield moment and plastic moment capacities and shape factor for major axis bending of the section shown in Fig. 1. Given $\mathrm{F}_{\mathrm{y}}=42 \mathrm{ksi}$


Fig: 1
(d) Select the lightest W section of A 992 ( $\mathrm{Fy}=50 \mathrm{ksi}$ ) steel to serve a column $25^{\prime}$ long to carry an axial compression load of 115 kips dead load and 150 kips live load in a braced structure, as shown in Fig. 2. Use ASD approach.


Fig: 2

